

Advising 4/16 - 4/27

Senior Design 2+2 1+3 3+1

FEW -
Building Design
Adv. Steel Design

Deep Foundations

Traffic Eng + Modeling
Urban Trans Systems

May 11th Fri 12:30

April 23 @ 6:00 PM

Senior Design Final Presentation

140 Smith

Analysis of Reinforced Concrete Beams

$$M_n = \underbrace{A_s f_y}_{T} (d - a/2)$$

$$\phi M_n \geq M_u$$

$$\phi = 0.90 \text{ bending}$$

$$R_n \geq \frac{M_u}{\phi b d^2}$$

Design Eq.

$$R_n = f(f_y, f'_c, \rho)$$

$f'_c, f_y, \rho_{design}, DL, LL$

$$\rho = \frac{A_s}{bd}$$

$$\rho_{min} \geq 200 / f_y$$

$$\geq 3 \sqrt{f'_c} / f_y$$

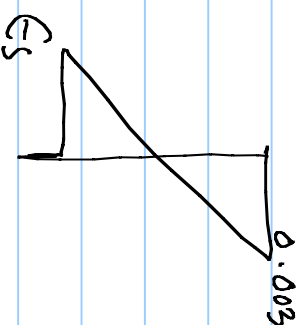
$f'_c > 4400 \text{ psi}$

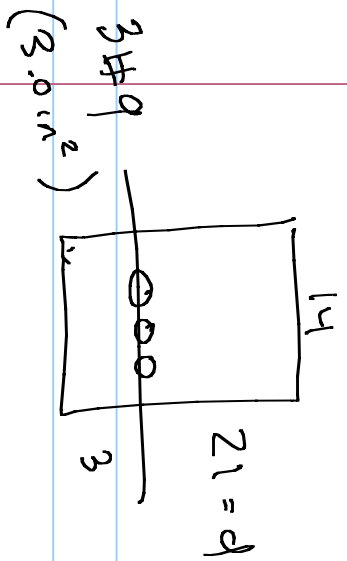
steel won't fracture
instantaneously
when concrete cracks
 M_{cr}

Ensure steel yields before concrete
in compression crushes

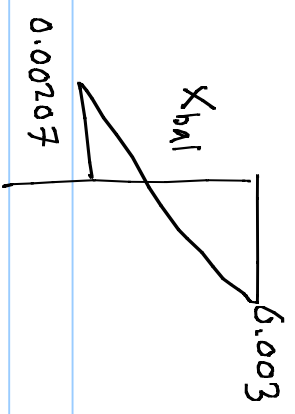
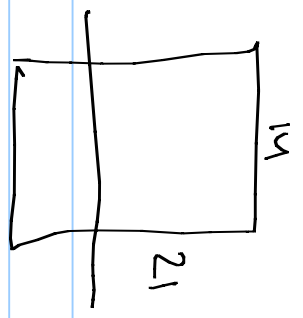
$$\epsilon_s > \epsilon_y \quad \text{when} \quad \epsilon_c = 0.003$$

ductile failure
tension controlled beam



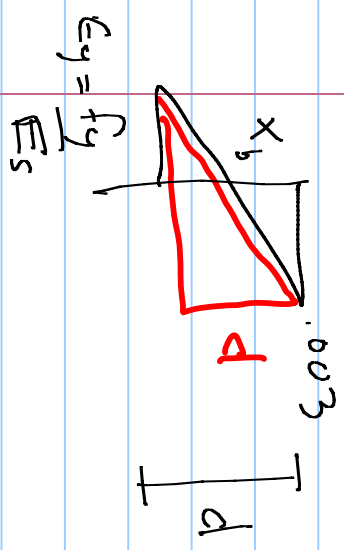


$f_c = 3 \text{ ksi}$
 $f_y = 60 \text{ ksi}$



Balanced Condition P_{bal}

% steel that will cause the concrete to crush ($\epsilon_c = 0.003$) at the same time that the steel (in tension) yields ($\epsilon_s = 0.00207$)



$$X_b = \frac{d}{\frac{0.003 + f_y / E_s}{0.00207}} = \frac{d}{\frac{0.003 + f_y / 29,000,000 \text{ psi}}{0.00207}}$$

$$X_b = \left[\frac{87,000}{87,000 + f_y} \right] d \quad f_y = 60 \text{ ksi}$$

$$C = T$$

$$0.85 f'_c b a = A_s b f_y$$

$$P = \frac{A_s}{bd}$$

$$a_{bal} = \frac{A_s f_y}{0.85 f'_c b} \bar{d}$$

$$a_{bal} = \frac{P_{bal} f_y \bar{d}}{0.85 f'_c}$$

$$X = \alpha / \beta_1 = \frac{P_{bal} f_y \bar{d}}{0.85 f'_c \beta_1}$$

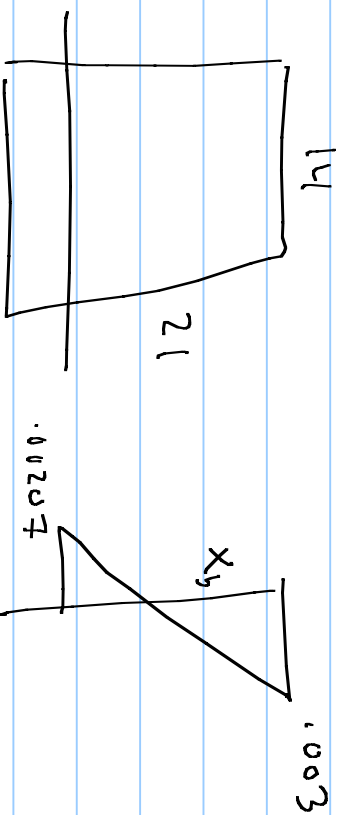
$$\left[\frac{87000}{87000 + f_y} \right] \bar{d} = \frac{P_{bal} f_y \bar{d}}{0.85 f'_c \beta_1}$$

$$P_{bal} = \frac{0.85 f'_c \beta_1}{f_y} \left[\frac{87,000}{87,000 + f_y} \right] \quad P_{bal} = f(f'_c, f_y)$$

$P_{des} < P_{bal}$ A.7 Table

Calculate A_{sb}

$$f'_c = 3 \text{ ksi}$$
$$f_y = 60 \text{ ksi}$$



$$\frac{x}{d} = \frac{d=21}{.003 + .00207}$$

$$x_b = 12.426 \text{ '}$$

$$C = T$$

$$a = \beta_1 X$$

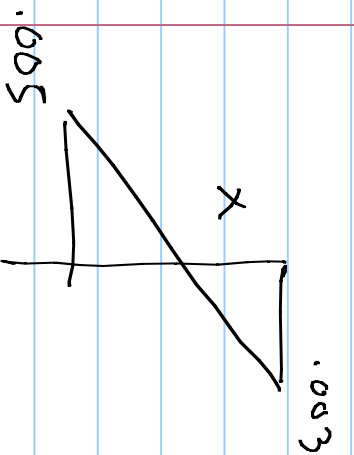
$$0.85 f'_c b a = A_s f_y$$

$$0.85(3)(14)(0.85)(12.426) = A_{sb}(60) \quad A_{sb} = 6.28 \text{ in}^2$$

$$\rho_b = 0.0214 \quad A_{sb} = \rho_b b d \quad \text{3\#9 bars (3.0)}$$

$$= 0.0214(14)(21) = 6.28 \text{ in}^2$$

$$P_{0.005}$$



$$\frac{x}{0.003} = \frac{d}{0.008} \quad x = \underline{\underline{\frac{3}{8}d}}$$

$$C = T$$

$$0.85 f' c h a = A_s f_y$$

$$a = \beta_1 x$$

$$0.85 f' c b \beta_1 x = A_s f_y$$

$$0.85 f' c b \beta_1 \left(\frac{3}{8}d\right) = A_s f_y$$

$$\frac{A_s}{b d} = 0.85 \beta_1 \left(\frac{3}{8}\right) = \rho_{0.005}$$

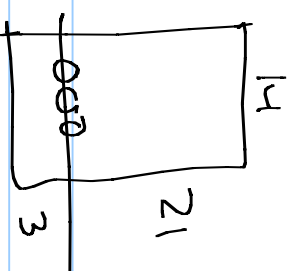
$$P \leq 0.175 P_{bal}$$

	$1/2 P_{bal}$	$P_{0.005}$	P_b
$\beta_1 = .85$	$0.21 f'_c / f_y$	$0.27 f'_c / f_y$	$0.43 f'_c / f_y$
$\beta_1 = .65$	$0.16 f'_c / f_y$	$0.21 f'_c / f_y$	$0.33 f'_c / f_y$

$$\rightarrow * \frac{1}{2} P_{bal} = P$$

$$0.18 f'_c / f_y$$

Check the Beam



$$f_c = 3 \text{ ksi}$$

$$f_y = 60 \text{ ksi}$$

$$\beta_1 = 0.85$$

$$C = T \rightarrow a = 5.04''$$

$$x = 5.93$$

$$M_n = 277.2 \text{ ft-k}$$

$$e_f = 0.0076$$

$$P_{bal} = 0.0214 \quad 2.1\%$$

$$A_{sb} = 6.28$$

$$P_{min} \geq 3\sqrt{f_c} / f_y = 0.0027$$

$$\geq 200 / f_y = 0.0033 \quad *$$

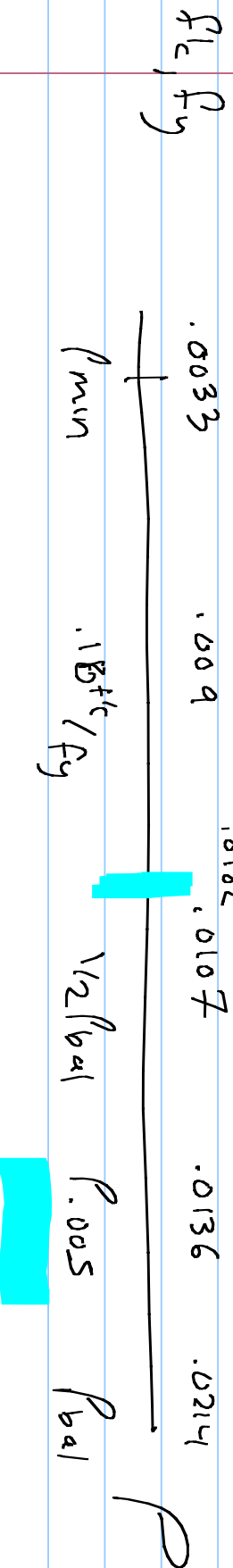
$$P_{bal} = 0.0214 \quad \cdot 85 \beta_1 f_c' / f_y \left(\frac{87000}{87000 + f_y} \right)$$

$$P_{0.05} = 0.0136 \quad \cdot 85 \beta_1 \frac{f_c'}{f_y} \left(\frac{3}{8} \right)$$

$$\frac{1}{2} P_b = 0.0107$$

$$0.18 f'_c / f_y = 0.009$$

$$P_{act} = \frac{3}{14 \times 21} = 0.0102 \quad \leftarrow \text{---} \text{---} \text{---}$$



Next Class

- Design a beam
- Practical spacing requirements